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THE STUDY OF THE INFLUENCE OF TRAFFIC INTENSITY AND TRAFFIC FLOW COMPOSITION ON THE CAPACITY OF MOTOR ROADS AND BRIDGE CROSSINGS

ДОСЛІДЖЕННЯ ВПЛИВУ ІНТЕНСИВНОСТІ РУХУ ТА СКЛАДУ ТРАНСПОРТНОГО ПОТОКУ НА ПРОПУСКНУ ЗДАТНІСТЬ АВТОМОБІЛЬНИХ ДОРІГ ТА МОСТОВИХ ПЕРЕХОДІВ



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Annotation: The study presents the results of the research of the impact of traffic intensity and traffic flow composition on the capacity of highways and bridge crossings based on mathematical and numerical modeling methods. The main factors affecting the capacity of highways and bridge crossings have been identified. The methodology for determining the level of traffic and capacity of highways approved by the State Service of Highways (now the Restoration Agency) was adopted as the main calculation basis for numerical modeling. Also, for calculations, updated statistical data, given in open literary sources, were used. 11 test options with different composition of the traffic flow were considered: from 0% to 100% of the share of passenger cars in the composition of the traffic flow. The average speed of the traffic flow was determined depending on the geometric parameters of the road - the radii of the horizontal curves in the plan and the longitudinal slope of the carriageway. It is shown that the average speed of the traffic flow increases with an increase in the value of the radius of the horizontal curve and with a decrease in the value of the longitudinal slope of the carriageway of the road, which is due to the improvement of traffic safety conditions and the possibility of moving vehicles at higher speeds. Also, the results of the numerical modeling showed an increase in the average speed of the traffic flow with an improvement in the level of coverage of the road (bridge crossing). It is shown that an increase in the share of cars in the traffic flow leads to an increase in the average speed of the entire traffic flow, which is explained by their more "compact" dimensions, compared to trucks, buses and road trains. The smallest practical carrying capacity of a highway or bridge crossing corresponds to a column of cars at the speed of free movement. The highest practical throughput corresponds to the traffic intensity at the minimum interval (optimal speed). Also, with an increase in the traffic flow of trucks and large-sized vehicles, the capacity of the road decreases. This determines the relevance of research in the direction of developing load models based on the actual parameters of heavy-duty rolling stock when determining the load-carrying capacity of highways and overpasses.

Key words: highway, bridge crossing, throughput, mathematical modeling.

Introduction. The carrying capacity of a road is one of the most important criteria that determines its overall efficiency and functioning. It characterizes the maximum number of vehicles that can safely and comfortably drive through a certain section of the road in a unit of time. A high carrying capacity indicates a road's ability to provide uninterrupted traffic flow, which is critical for reducing congestion, improving road safety, and improving transport logistics in the region.

The carrying capacity of the highway (bridge crossing) depends on a number of factors:

1. Geometric parameters of the road. The roadway width, number of lanes, turning radius, gradients and road profile affect the maximum number of vehicles that can pass in a given time.

2. Road surface quality. The condition and type of road surface affect the speed and safety of transport, which directly affects the throughput.

3. Mode of movement. The presence and regulation of traffic lights, road signs, pedestrian crossings, as well as speed limits affect the level of continuity and speed of movement.

4. Transport flow. The intensity of traffic, the composition of the transport flow, its average speed and density affect the carrying capacity of the road.

5. Weather conditions. Rain, snow, ice can significantly reduce traffic capacity due to the need to reduce speed and increase the distance between vehicles.

6. Presence of emergency situations or repair work. Any obstacles on the road, including road accidents, repair work or other unforeseen situations, can significantly reduce its capacity, creating traffic jams and slowing down traffic.

7. Traffic organization. Proper traffic planning and management, including interchanges, flyovers, and roundabouts, can increase capacity by reducing congestion and improving traffic flow.

Studies of the carrying capacity of highways and bridge crossings were carried out by such scientists as: V.M. Sidenko, E.M. Lobanov, V.V. Silyanov, F. Hait, D. Drew, A. Makki, T. Nguyen, J. Ren, D. Al-Jumeily, W. Hurst, Birulya O.K., Khomyak Y.V., Gukov M.I., Tribunskyi V.M., Keroglu L.A., Kaluzhskiy Y.A., Vulis D.A., Palchyk A.M., Neizvestna N.V., Dodukh K.M. and many others.

In the study [1], the authors analyzed the theoretical and methodological approaches to determining

the carrying capacity of highways and bridges, and also established the dependence of the traffic intensity on the approaches to the bridge crossing on the longitudinal slope and the composition of the traffic flow.

In the study [2], an analysis of normative documents regulating the maximum permissible values of geometric and weight parameters of motor vehicles was carried out; the need to improve the regulatory and technical base in the direction of determining the actual loads on existing road bridges, taking into account possible defects of the surface of the covering, is substantiated.

The purpose of the study is to study the impact of traffic intensity and the composition of the traffic flow on the capacity of highways and bridge crossings.

The object of the study is traffic flows moving along highways and overpasses.

The subject of the study is the carrying capacity of highways and bridge crossings depending on the intensity of traffic and the composition of the traffic flow.

Methods of the research. Theoretical-analytical, mathematical and numerical modeling.

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Presentation of the main material. To study the influence of the composition of the traffic flow and road conditions on the carrying capacity of a highway or bridge crossing, a series of computational experiments was conducted based on the methodology M 218 - 02070915 - 674: 2010 "Methodology for determining the level of traffic and carrying capacity of highways" [3].

The following assumptions were made for numerical modeling: the category of the section of the road is II; traffic intensity – 3500-4000 cars/day.

For the calculation, 11 test variants with different variations in the composition of the traffic flow were accepted (Table 1).

Table 1 – Input data for numerical simulation

Таблиця 1 – Вихідні дані для проведення числового моделювання

| Test variants | The composition of the traffic flow by types of cars, % | | | | |
|---------------|---|--------|-------|-------------|--------|
| | Passenger cars | Trucks | Buses | Road trains | Amount |
| Variant 1 | 0 | 80 | 10 | 10 | 100 |
| Variant 2 | 10 | 70 | 10 | 10 | 100 |
| Variant 3 | 20 | 60 | 10 | 10 | 100 |
| Variant 4 | 30 | 50 | 10 | 10 | 100 |
| Variant 5 | 40 | 40 | 10 | 10 | 100 |
| Variant 6 | 50 | 30 | 10 | 10 | 100 |
| Variant 7 | 60 | 20 | 10 | 10 | 100 |
| Variant 8 | 70 | 10 | 10 | 10 | 100 |
| Variant 9 | 80 | 0 | 10 | 10 | 100 |
| Variant 10 | 90 | 0 | 5 | 5 | 100 |
| Variant 11 | 100 | 0 | 0 | 0 | 100 |

According to [3], the average speed of the traffic flow on the j-th section of the road is recommended to be determined as the minimum of the average speeds that depend on the following parameters: traffic intensity (V_N); the radius of the horizontal curve (V_R); values of the longitudinal slope (V_j); indicator of evenness of coverage (V_{eq}); speed limit values (V_{lim}); values of the average speed of free movement (V_{free}).

$$\bar{V}_j = \min(V_N, V_R, V_{eq}, V_{lim}, V_{free}) \tag{1}$$

One of the most important parameters that determines the nature of the transport flow and, accordingly, the capacity of the road is the average speed, which depends on the composition of the transport flow and can be determined as an average statistical value according to the formula [3]:

$$V_{free} = V_{pass} \times \alpha + V_{tr} \times \beta + V_{bus} \times \gamma + V_{r.tr} \times \rho, \tag{2}$$

where V_{pass} , V_{tr} , V_{bus} , $V_{r.tr}$ – respectively, average speeds of free movement of cars, trucks, buses and road trains, km/h;

$\alpha, \beta, \gamma, \rho$ – shares of passenger cars, trucks, buses and road trains in the traffic flow, respectively.

Values of average speeds of different types of cars included in (2), as a rule, are determined on the basis of statistical generalization of experimental measurement data. It should be noted that the average speed of cars of different types is not a constant value, but can change over time, depending on the composition of the traffic flow and the technical characteristics of cars of one or another type present in it. For the numerical simulation, the data on the average speed of the cars, given in the methodology [3] and in the work [4] (Table 2), were accepted.

Table 2 – Average speeds of different types of cars

Таблиця 2 – Середні швидкості руху різних типів автомобілів

| Road category | Number of traffic lanes | Average speed, km/h | | | | | | | |
|---------------|-------------------------|---------------------|-------|--------|-------|-------|-------|-------------|-------|
| | | Passenger cars | | Trucks | | Buses | | Road trains | |
| | | [3] | [4] | [3] | [4] | [3] | [4] | [3] | [4] |
| I a | 6 | 85 | 91,13 | 65 | 75,7 | 73,4 | 77,5 | 70,4 | 81,03 |
| I б | 4 | 83,4 | 88,04 | 64,7 | 75,77 | 68,3 | 74,61 | 66,1 | 80 |
| II | 2 | 76,4 | 84,29 | 62,6 | 71,9 | 66,0 | 71,5 | 63,0 | 72,93 |
| III | 2 | 70,6 | 79,72 | 57,8 | 67,06 | 61,0 | 69,33 | 57,6 | 71,11 |
| IV | 2 | 70,4 | 75,83 | 57,1 | 64,08 | 61,0 | 67,03 | 57,2 | 68,75 |

The results of calculating the values of the average speeds of free movement depending on the composition of the traffic flow are shown in Table 3 and Fig. 1.

Table 3– Estimated values of average speeds of free movement

Таблиця 3 – Розрахункові значення середніх швидкостей вільного руху

| Test variants | The share of passenger cars in the flow, % | The average free flow speed, km/h | | Test variants | The share of passenger cars in the flow, % | The average free flow speed, km/h | |
|---------------|--|-----------------------------------|------------------------------------|---------------|--|-----------------------------------|------------------------------------|
| | | according to the method [3] | according to experimental data [4] | | | according to the method [3] | according to experimental data [4] |
| Variant 1 | 0 | 62,98 | 71,963 | Variant 7 | 60 | 71,26 | 79,397 |
| Variant 2 | 10 | 64,36 | 73,202 | Variant 8 | 70 | 72,64 | 80,636 |
| Variant 3 | 20 | 65,74 | 74,441 | Variant 9 | 80 | 74,02 | 81,875 |
| Variant 4 | 30 | 67,12 | 75,68 | Variant 10 | 90 | 75,21 | 83,0825 |
| Variant 5 | 40 | 68,5 | 76,919 | Variant 11 | 100 | 76,4 | 84,29 |
| Variant 6 | 50 | 69,88 | 78,158 | | | | |

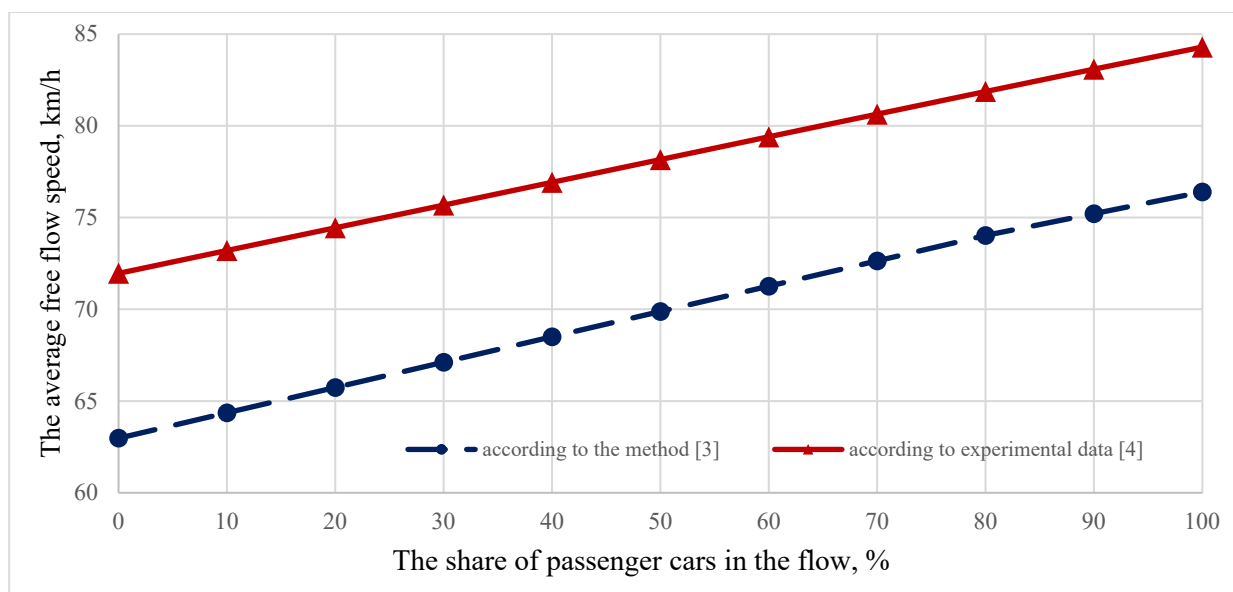


Figure 1 – Dependence of the average speeds of the free movement of the transport flow on the share of passenger cars in it

Рисунок 1 – Залежність середніх швидкостей вільного руху транспортного потоку від частки легкових автомобілів у ньому

The analysis of Table 3 and Figure 1 allows us to conclude that the average speed of the free movement of the traffic flow increases with the increase in the share of passenger cars in its composition, which is due to their higher dynamic characteristics.

According to the method [3], the average speed of the traffic flow depends on the radius of the horizontal curve of the road and is determined by the formulas:

$$V_R = \begin{cases} \frac{R^{0.25} \times V_0}{2}, & R < 600\text{м}; \\ V_{free}, & R \geq 600\text{м} \end{cases}, \quad (3)$$

where V_0 – speed corresponding to the maximum throughput, km/h.

To study the influence of the radii of horizontal curves on the average speed of the traffic flow, a numerical experiment was conducted with the variation of the radius $R = 50 - 600$ m with a step of 50 m. For the calculation, the composition of the traffic flow with a share of passenger cars of 30% was adopted. The results of the calculations are shown in Table 4 and Figure 2.

Table 4– Estimated values of average speeds of traffic flow at different radii of horizontal curves

Таблиця 4 – Розрахункові значення середніх швидкостей транспортного потоку при різних радіусах горизонтальних кривих

| R | 50 | 100 | 150 | 200 | 250 | 300 | 350 | 400 | 500 | 600 |
|-------|------|------|------|------|------|------|------|------|------|------|
| V_R | 33,2 | 39,5 | 43,7 | 47,0 | 49,7 | 52,0 | 54,1 | 55,9 | 59,1 | 61,9 |

The analysis of Table 4 and Figure 2 allows us to conclude that the average speed of the traffic flow increases with an increase in the radius of the horizontal curve, which is due to the improvement of traffic safety conditions and, accordingly, the ability to move vehicles with higher average speeds.

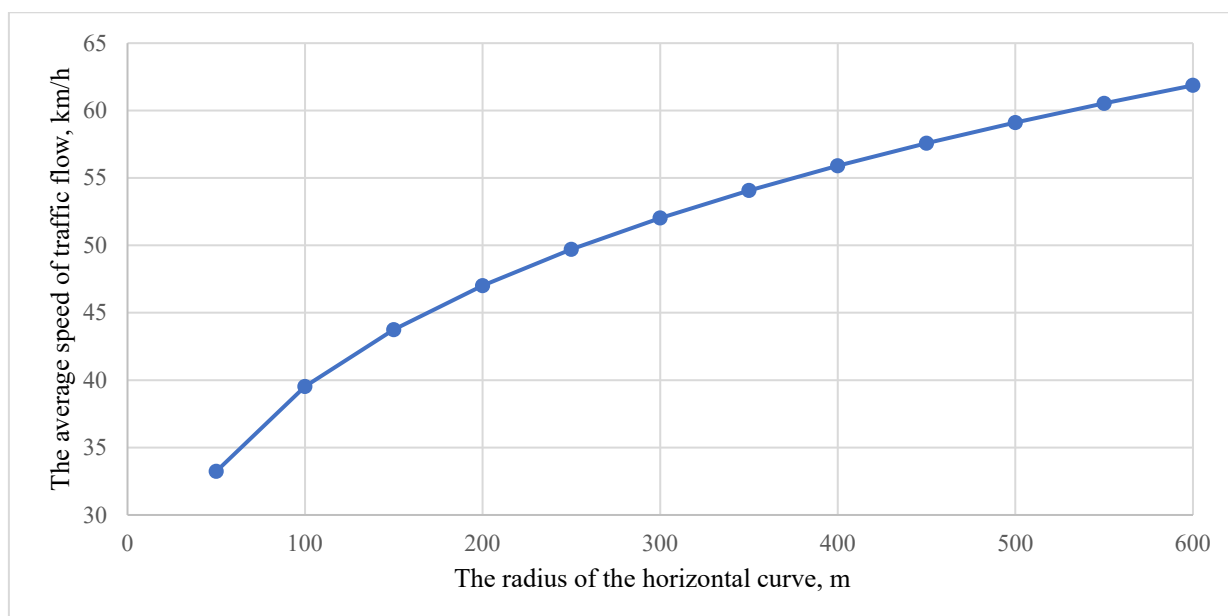


Figure 2 – Dependence of the average traffic flow speeds on the radii of horizontal curves

Рисунок 2 – Залежність середніх швидкостей транспортного потоку від радіусів горизонтальних кривих

The average speed of the traffic flow also depends on the longitudinal profile of the road and is determined by the formula [3]:

$$V_i = V_{free} \times \frac{0,02 \frac{V_0}{V_{free}}}{i \frac{V_0}{V_{free}}}, \quad (4)$$

where i – longitudinal slope in fractions of a unit.

The average speed on sections of highways with slopes is equal to the average speed of free traffic [3]:

$$V_i = V_{free}, \quad (5)$$

To study the influence of the longitudinal slope of the highway on the average speed of the traffic flow, the calculation was carried out according to formula (3) with varying values of the longitudinal slope i from 0.008 to 0.06. For the calculation, the composition of the traffic flow with a share of passenger cars of 30% was adopted. The results of the calculations are shown in Table 5 and Figure 3.

Table 5– Estimated values of the average traffic flow speeds for different longitudinal slopes of the highway

Таблиця 5 – Розрахункові значення середніх швидкостей транспортного потоку за різних поздовжніх похилів автомобільної дороги

| Longitudinal slope, <i>fractions of a unit</i> | 0,008 | 0,01 | 0,02 | 0,03 | 0,04 | 0,05 | 0,06 |
|---|-------|------|------|------|------|------|------|
| Average speeds of the traffic flow according to the method [3], <i>km/h</i> | 94,4 | 86,9 | 67,1 | 57,7 | 51,8 | 47,7 | 44,6 |
| The average speeds of the traffic flow, determined on the basis of experimental data [4], <i>km/h</i> | 102,4 | 95,2 | 75,7 | 66,2 | 60,2 | 55,9 | 52,6 |

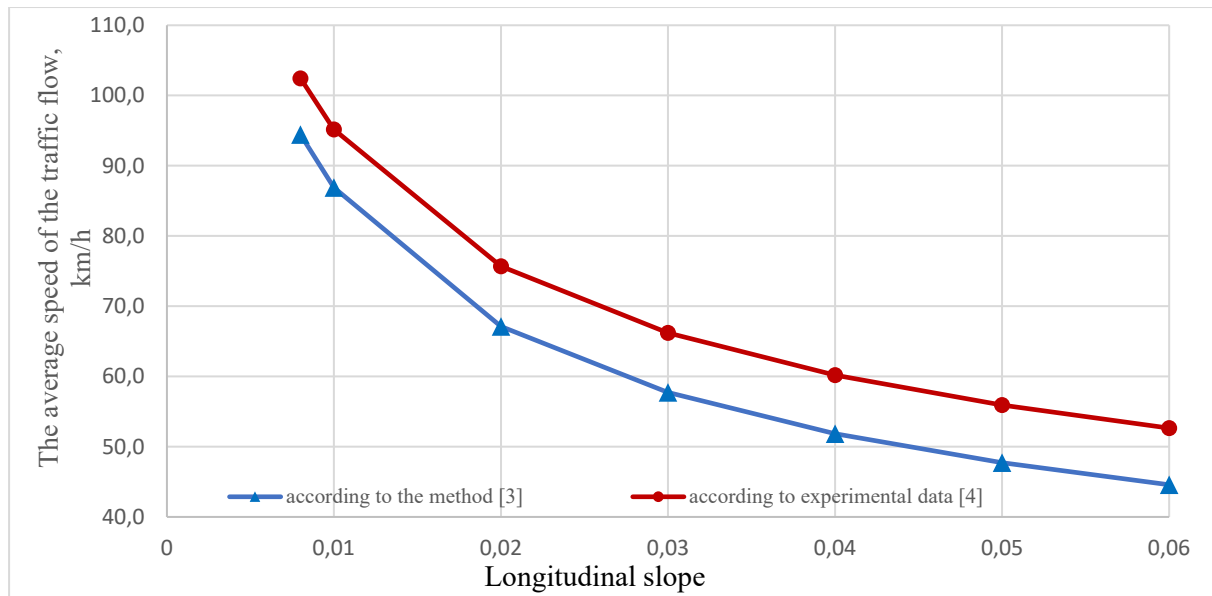


Figure 3 – Dependence of the average speed of the traffic flow on the longitudinal slope of the road
Рисунок 3– Залежність середніх швидкостей транспортного потоку від поздовжнього похилу дороги

The analysis of Table 5 and Figure 3 allows us to conclude that the average speed of the traffic flow is increasing with a decrease in the value of the longitudinal slope of the carriageway of the road, which is determined by the conditions of safety and the ability of vehicles to move.

The average speed depending on the evenness of the coating is determined by the formula [3]:

$$V_{eq} = V_{free} \times \frac{50^{V_0/V_{free}}}{P^{V_0/V_{free}}}, \quad (6)$$

where P – the indicator of the tachometer, cm/km.

In the car-road subsystem, the permissible speed of the traffic flow V_p can be determined from the condition of preventing excessive oscillations of the car when it moves on an uneven surface [1]:

$$V_p = \frac{850}{\sqrt{S}}, \quad (7)$$

where S – value characterizing the total amplitude of oscillations of a reference car on a road section 1 km long.

Depending on the marginal equality s_{me} , it is possible to determine V_p . According to the current regulations, the longitudinal evenness of the road surface is assessed by the indicators of equality in linear units per unit length of the road section [6, 7]:

$$K_{eq} = \frac{S_{me}}{S_f}, \quad (8)$$

where s_{me} – the maximum permissible evenness of the road surface, according to the profilometric method, cm/km or according to the index of the pushometer, cm/km; S_f – the actual unevenness of the road surface, which is estimated by the profilometric method, m/km or by the index of the pushometer, cm/km.

The maximum allowable evenness of the road surface is assigned depending on the level of requirements for the operational condition of highways, according to Table 3 [7].

To study the impact of the evenness of the road surface on the average speed of the traffic flow, the calculation was carried out according to formulas (6) – (8). For the calculation, the normative indicators of the coverage equality according to [6] were used. Also, the calculation was carried out for some values of equality indicators, which are not normalized. For the calculation, the composition of the transport flow with the share of passenger cars – 30% was taken. The results of the calculations are given in Table 6 and Figure 4.

Table 6– Calculated values of the average speeds of the traffic flow at different values of the evenness of the road surface

Таблиця 6 – Розрахункові значення середніх швидкостей транспортного потоку при різних значеннях рівності покриття автомобільної дороги

| Indicator | Not normalized indicators of evenness | | | Standard indicators of the evenness of the road surface | | | | | | | |
|--|---------------------------------------|-------|------|---|------|------|------|------|------|------|------|
| | | | | The level of requirements for highways | | | | | | | |
| | 1 | 2 | 3 | 4 | A | Б | В | Г | | | |
| Road surface evenness indicator, no more than <i>cm/km</i> | 50 | 70 | 80 | 100 | 120 | 170 | 240 | 100 | 120 | 240 | 260 |
| Average speeds of the traffic flow according to the method [3], <i>km/h</i> | 67,1 | 59,2 | 56,3 | 51,8 | 48,4 | 42,5 | 37,4 | 51,8 | 48,4 | 37,4 | 36,3 |
| Average speeds of traffic flow according to experimental data [4], <i>km/h</i> | 75,7 | 67,7 | 64,8 | 60,2 | 56,7 | 50,5 | 45,1 | 60,2 | 56,7 | 45,1 | 43,9 |
| The calculated value of the permissible speed in the car-road subsystem, <i>km/h</i> | 120,2 | 101,6 | 95,0 | 85,0 | 77,6 | 65,2 | 54,9 | 85,0 | 77,6 | 54,9 | 52,7 |

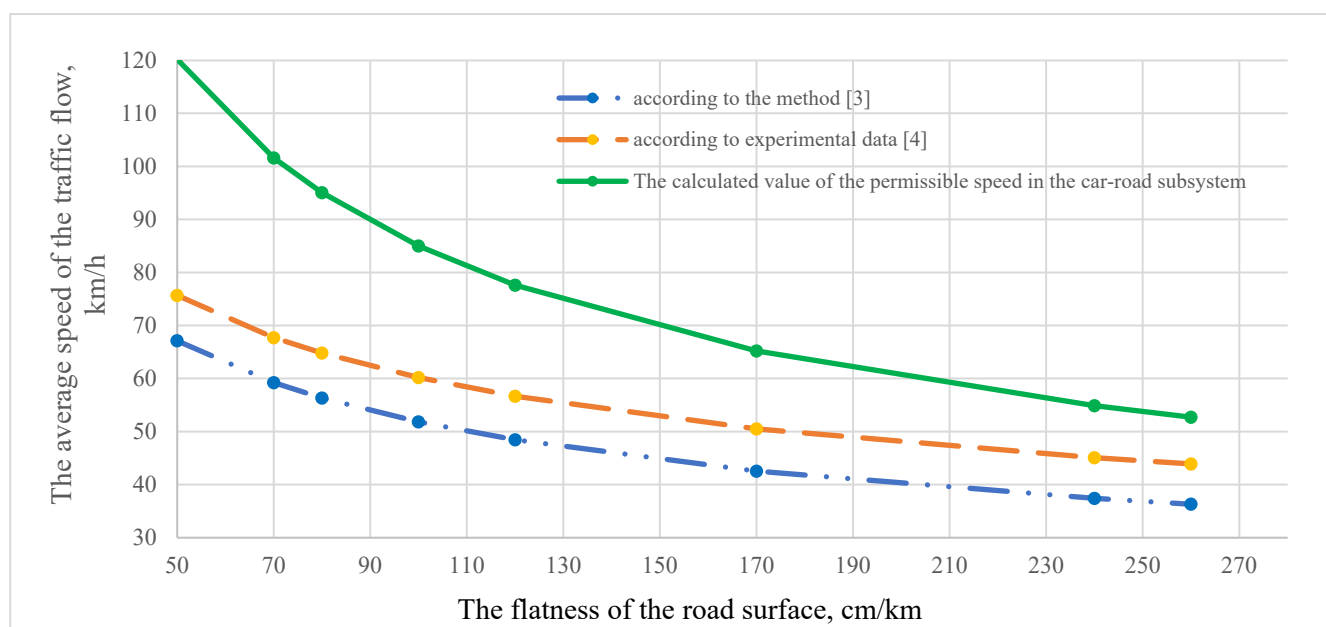


Figure 4 – Dependence of the average speeds of the traffic flow on the evenness of the road surface
Рисунок 4 – Залежність середніх швидкостей транспортного потоку від рівності покриття дороги

The analysis of Table 6 and Figure 4 allows us to conclude that the average speed of the traffic flow is increasing with the improvement of the level of coverage of the road, which is due to the improvement of traffic safety conditions and the ability to drive vehicles at higher speeds.

The average speed depending on the intensity of traffic is determined by the formula [3]:

$$V_N = (10,47 - \frac{N}{q_{\max}}), \tag{9}$$

where N – traffic intensity, cars/hour.;

q_{\max} – the maximum density of the traffic flow, cars/km, which is determined by the formula [3] :

$$q_{\max} = \frac{1000}{1 + L_a}, \tag{10}$$

where L_a – the average length of the car in the traffic flow, m, determined by the formula [3]:

$$L_a = 4,2 \times \alpha + 7,0 \times \beta + 10,5 \times \gamma + 12,0 \times \rho, \tag{11}$$

where 4,2; 7,0; 10,5; 12,0 – according to the length of cars, trucks, buses, road trains (according to the methodology [3], as of 2010.

In the study [5], based on experimental studies of transport flows in Ukraine in 2020, the data on the lengths of different types of cars were clarified: cars - 3.6 m, trucks - 6.2 m, road trains - 14.4 m, and buses - 11, 9 m.

As part of the work, the maximum traffic density (in one lane) was determined according to formulas (9) - (11) depending on the composition of the traffic flow. For the calculation, data on the overall dimensions of various types of cars, given in the methodology M 218 – 02070915 – 674: 2010 [3] and specified data given in the work [5], were used. The results of the calculations are given in Table 7 and Figure 5.

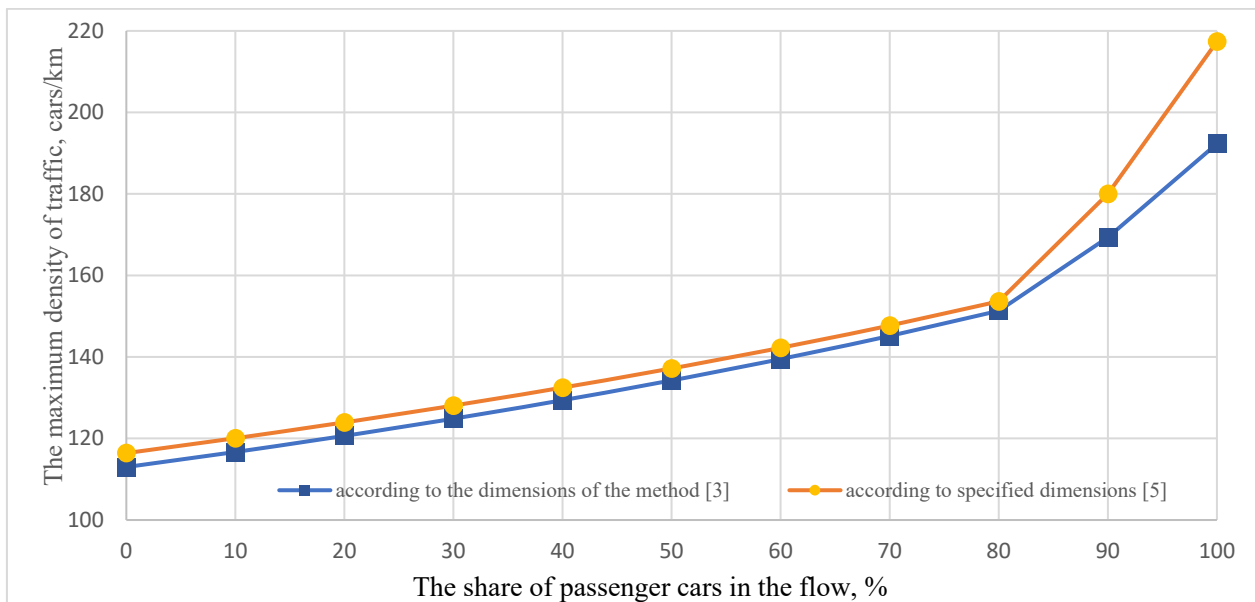


Figure 5 – Dependence of the maximum density of the traffic flow on its composition and weighted average size

Рисунок 5 – Залежність максимальної щільності транспортного потоку від його складу і середньозваженого габариту

Table 7 – Estimated values of the maximum density of the traffic flow depending on the composition of the transport flow

Таблиця 7 – Розрахункові значення максимальної щільності руху транспортного потоку залежно від складу транспортного потоку

| Test variants | The composition of the traffic flow, % | | | | 2010 year, in accordance [3] | | 2020 year, in accordance [5] | |
|---|--|--------|-------|-------------|------------------------------|------------------|------------------------------|------------------|
| | Passenger cars | Trucks | Buses | Road trains | La | q _{max} | La | q _{max} |
| Variant 1 | 0 | 80 | 10 | 10 | 7,85 | 112,99 | 7,59 | 116,414 |
| Variant 2 | 10 | 70 | 10 | 10 | 7,57 | 116,69 | 7,33 | 120,048 |
| Variant 3 | 20 | 60 | 10 | 10 | 7,29 | 120,63 | 7,07 | 123,916 |
| Variant 4 | 30 | 50 | 10 | 10 | 7,01 | 124,84 | 6,81 | 128,041 |
| Variant 5 | 40 | 40 | 10 | 10 | 6,73 | 129,37 | 6,55 | 132,45 |
| Variant 6 | 50 | 30 | 10 | 10 | 6,45 | 134,23 | 6,29 | 137,174 |
| Variant 7 | 60 | 20 | 10 | 10 | 6,17 | 139,47 | 6,03 | 142,248 |
| Variant 8 | 70 | 10 | 10 | 10 | 5,89 | 145,14 | 5,77 | 147,71 |
| Variant 9 | 80 | 0 | 10 | 10 | 5,61 | 151,29 | 5,51 | 153,61 |
| Variant 10 | 90 | 0 | 5 | 5 | 4,905 | 169,35 | 4,555 | 180,018 |
| Variant 11 | 100 | 0 | 0 | 0 | 4,2 | 192,31 | 3,6 | 217,391 |
| Accepted overall dimensions of cars of various types, m | | | | | | | | |
| according to the data of 2010 [3] | 4,2 | 7 | 10,5 | 12 | | | | |
| according to the data of 2020 [5] | 3,6 | 6,2 | 11,9 | 14,4 | | | | |

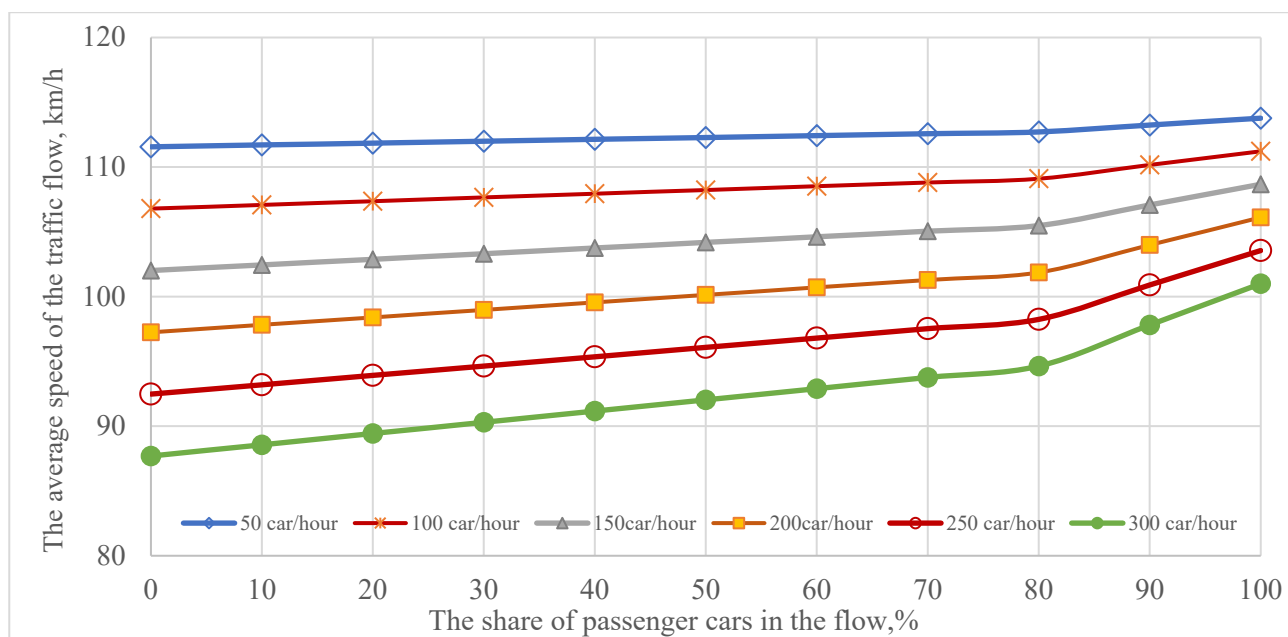


Figure 6 – The dependence of the average speed of the traffic flow on the intensity of traffic and the composition of the traffic flow

Рисунок 6 – Залежність середньої швидкості транспортного потоку від інтенсивності руху і складу транспортного потоку

Also, according to formula (9), the average speed of the traffic flow (in one lane) was determined depending

on the traffic density and composition of the traffic flow. For the calculation, data on the overall dimensions of various types of cars, given in [5], were used. The results of the calculations are shown in Figure 6.

The analysis of Table 6 and Figure 4 allows us to make a conclusion about the increase in traffic density with the increase in the share of cars in it, which is explained by their more "compact" dimensions compared to trucks, buses and road trains. Also, as the intensity of traffic flow increases, its average speed decreases. The average speed of the traffic flow and the capacity of the highway (one lane) depending on the average speed of the traffic flow are determined by the formula [3]:

$$P = \frac{1000 \times V}{(d_{\min} + L_a) \times e^{\frac{V}{V_0}}}, \quad (12)$$

where v – average speed of the traffic flow, km/h. (established by direct measurements or by calculations; d_{\min} – minimum safe distance between cars in a traffic jam, m ($d_{\min} = 1\text{ м}$);

L_a – average length of the car in the traffic flow, m; V_0 – the speed corresponding to the maximum throughput, km/h (most often accepted $V_0 = 25$ km/h).

For the calculation, data on the overall dimensions of various types of cars, given in [5], were used. The results of the calculations are shown in Table 8 and Figure 7.

The analysis of Table 8 and Figure 7 allows us to conclude that the practical carrying capacity of the road varies from the smallest to the largest. The smallest practical throughput corresponds to the column traffic of cars at the speed of free movement. The highest practical throughput corresponds to the traffic intensity at the minimum interval (optimal speed). Also, with an increase in the traffic flow of trucks and large-sized vehicles, the capacity of the road decreases.

Table 8– Estimated values of the road capacity depending on the average speed of the traffic flow, the intensity of traffic and the composition of the traffic flow

Таблиця 8 – Розрахункові значення пропускної здатності автомобільної дороги залежно від середньої швидкості транспортного потоку від інтенсивності руху і складу транспортного потоку

| Test variants | A share of passenger cars, % | Throughput capacity (one lane) at the average speed of the traffic flow, km/h | | | | | | | | |
|---------------|------------------------------|---|--------|--------|--------|--------|--------|-------|-------|-------|
| | | 5 | 10 | 25 | 30 | 50 | 60 | 70 | 80 | 90 |
| Variant 1 | 0 | 476,6 | 780,3 | 1070,7 | 1051,9 | 787,7 | 633,7 | 495,5 | 379,6 | 286,3 |
| Variant 2 | 10 | 491,4 | 804,7 | 1104,1 | 1084,7 | 812,3 | 653,4 | 511,0 | 391,5 | 295,2 |
| Variant 3 | 20 | 507,3 | 830,6 | 1139,7 | 1119,7 | 838,5 | 674,5 | 527,5 | 404,1 | 304,7 |
| Variant 4 | 30 | 524,2 | 858,3 | 1177,6 | 1157,0 | 866,4 | 696,9 | 545,0 | 417,5 | 314,9 |
| Variant 5 | 40 | 542,2 | 887,8 | 1218,1 | 1196,8 | 896,3 | 720,9 | 563,8 | 431,9 | 325,7 |
| Variant 6 | 50 | 561,5 | 919,5 | 1261,6 | 1239,5 | 928,2 | 746,6 | 583,9 | 447,3 | 337,3 |
| Variant 7 | 60 | 582,3 | 953,5 | 1308,2 | 1285,3 | 962,6 | 774,3 | 605,5 | 463,9 | 349,8 |
| Variant 8 | 70 | 604,7 | 990,1 | 1358,5 | 1334,7 | 999,5 | 804,0 | 628,8 | 481,7 | 363,2 |
| Variant 9 | 80 | 628,8 | 1029,7 | 1412,7 | 1388,0 | 1039,4 | 836,1 | 653,9 | 500,9 | 377,7 |
| Variant 10 | 90 | 736,9 | 1206,7 | 1655,6 | 1626,6 | 1218,1 | 979,9 | 766,3 | 587,0 | 442,7 |
| Variant 11 | 100 | 889,9 | 1457,2 | 1999,3 | 1964,3 | 1471,0 | 1183,3 | 925,4 | 708,9 | 534,6 |

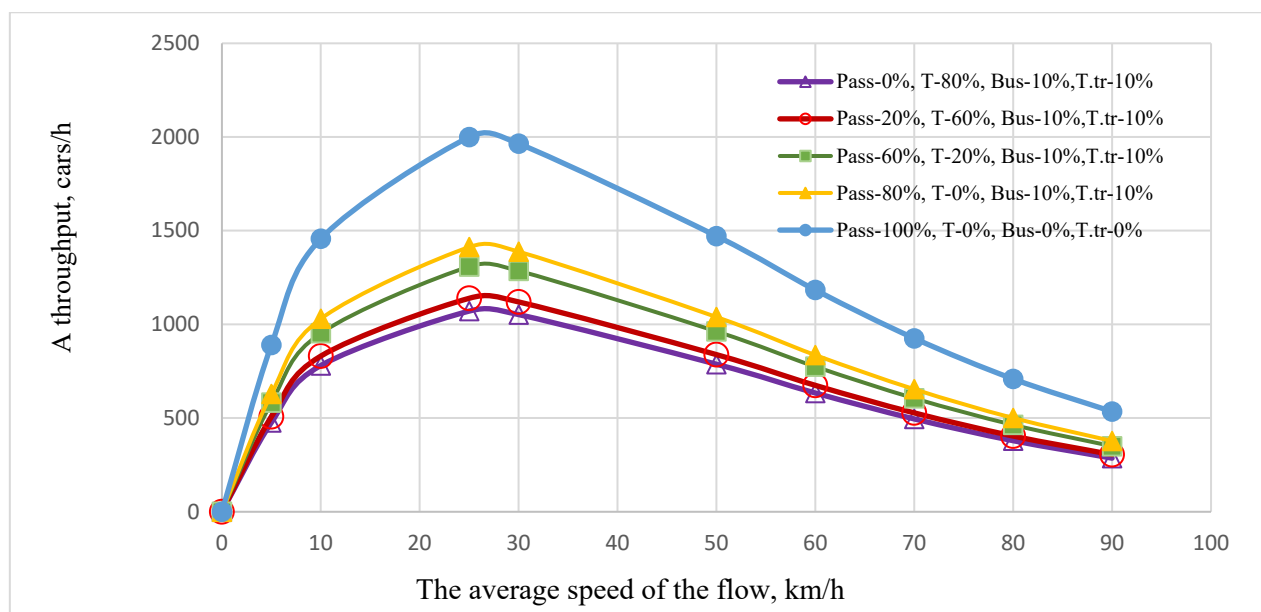


Figure 7 – Dependence of a road capacity on average speed and composition of a traffic flow
Рисунок 7 – Залежність пропускної здатності автомобільної дороги від середньої швидкості і складу транспортного потоку

Conclusions. The study of the capacity of highways and bridge crossings is an important tool for ensuring the safety, efficiency and sustainable development of the transport system as a whole. Also, the carrying capacity of a road is the most important indicator when designing its transverse profile and geometric elements. Based on the results of numerical modeling, it can be concluded that the main factor on which the maximum traffic intensity and, accordingly, the capacity of highways depends is the average speed of the traffic flow, which in its turn depends on the average speed of free traffic and its decrease depending on the composition of the traffic flow, the radius of horizontal curves and the longitudinal slope of the road.

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ДОСЛІДЖЕННЯ ВПЛИВУ ІНТЕНСИВНОСТІ РУХУ ТА СКЛАДУ ТРАНСПОРТНОГО ПОТОКУ НА ПРОПУСКНУ ЗДАТНІСТЬ АВТОМОБІЛЬНИХ ДОРІГ ТА МОСТОВИХ ПЕРЕХОДІВ

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Анотація. В роботі представлено результати дослідження впливу інтенсивності руху та складу транспортного потоку на пропускну здатність автомобільних доріг та мостових переходів на основі методів математичного та числового моделювання. Визначено основні фактори, що впливають на пропускну здатність автомобільних доріг та мостових переходів. За основну розрахункову базу для проведення числового моделювання прийнято методика визначення рівня завантаженості та пропускну здатності автомобільних доріг, затверджену Державною службою автомобільних доріг (нині – Агенством відновлення). Також для проведення розрахунків використано уточнені актуальні статистичні дані, наведені у відкритих літературних джерелах. Для проведення числового експерименту прийнято 11 тестових варіантів з різним складом транспортного потоку: від 0% до 100% частки легкових автомобілів у складі транспортного потоку. Визначено середню швидкість транспортного потоку залежно від геометричних параметрів автомобільної дороги – радіусів горизонтальних кривих в плані та поздовжнього похилу проїзної частини. Показано зростання середньої швидкості транспортного потоку зі збільшенням значення радіуса горизонтальної кривої та зі зменшенням значення поздовжнього похилу проїзної частини автомобільної дороги, що обумовлюється покращенням умов безпеки руху і можливостями руху транспортних засобів з більш високими швидкостями. Також результати числового моделювання показали зростання середньої швидкості транспортного потоку із покращенням показника рівності покриття автомобільної дороги (мостового переходу). Показано, що зростання частки легкових автомобілів у транспортному потоці приводить до зростання середньої швидкості руху всього транспортного потоку, що пояснюється їх більш «компактними» габаритами, порівняно із вантажівками, автобусами і автопотягами. Найменша практична пропускну здатність автомобільної дороги або мостового переходу відповідає колонному руху автомобілів при швидкості вільного руху. Найбільша практична пропускну здатність відповідає інтенсивності руху при мінімальному інтервалі (оптимальній швидкості). Також при збільшенні у складі транспортного потоку вантажних і великогабаритних автомобілів, пропускну здатність дороги зменшується. Це обумовлює актуальність досліджень у напрямку розробки моделей навантажень за фактичними параметрами великовагового рухомого складу при визначенні вантажно-пропускну здатності автомобільних доріг і мостових переходів.

Ключові слова: автомобільна дорога, мостовий перехід, пропускну здатність, математичне моделювання, чилвий експеримент.

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